

1200 New Jersey Ave., SE Washington, D.C. 20590

January 30, 2017

In Reply Refer To: HSST-1/B-274

Mr. Aris Stathopoulos, P.E. New York Metropolitan Transportation Authority (MTA) Bridges and Tunnels 2 Broadway, 22nd Floor New York, NY, 10004

Dear Mr. Stathopoulos:

1700 for villagers Asia i Rê-Kilessandor († 16. a. 1960)

This letter is in response to your November 29, 2016 request for the Federal Highway Administration (FHWA) to review a roadside safety device, hardware, or system for eligibility for reimbursement under the Federal-aid highway program. This FHWA letter of eligibility is assigned FHWA control number B-274 and is valid until a subsequent letter is issued by FHWA that expressly references this device.

Decision

The following devices are eligible, with details provided in the form which is attached as an integral part of this letter:

• Triborough Bridge and Tunnel Authority (TBTA) Bridge Rail

Scope of this Letter

To be found eligible for Federal-aid funding, new roadside safety devices should meet the crash test and evaluation criteria contained in the American Association of State Highway and Transportation Officials' Manual for Assessing Safety Hardware (MASH). However, the FHWA, the Department of Transportation, and the United States Government do not regulate the manufacture of roadside safety devices. Eligibility for reimbursement under the Federal-aid highway program does not establish approval, certification or endorsement of the device for any particular purpose or use.

This letter is not a determination by the FHWA, the Department of Transportation, or the United States Government that a vehicle crash involving the device will result in any particular outcome, nor is it a guarantee of the in-service performance of this device. Proper manufacturing, installation, and maintenance are required in order for this device to function as tested.

This finding of eligibility is limited to the crashworthiness of the system and does not cover other structural features, nor conformity with the Manual on Uniform Traffic Control Devices.

Eligibility for Reimbursement

Based solely on a review of crash test results and certifications submitted by the manufacturer, and the crash test laboratory, FHWA agrees that the device described herein meets the crash test and evaluation criteria of the American Association of State Highway and Transportation Officials' Manual for Assessing Safety Hardware (MASH). Therefore, the device is eligible for reimbursement under the Federal-aid highway program if installed under the range of tested conditions.

Name of system: TBTA Bridge Rail Type of system: Bridge Barrier Test Level: MASH Test Level 5 (TL5) Testing conducted by: Texas A&M Transportation Institute (TamTI) Date of request: November 4, 2016 Date initially acknowledged: November 6, 2016 Date of completed package: November 29, 2016

FHWA concurs with the recommendation of the accredited crash testing laboratory as stated within the attached form.

Full Description of the Eligible Device

The device and supporting documentation, including reports of the crash tests or other testing done, videos of any crash testing, and/or drawings of the device, are described in the attached form.

Notice

If a manufacturer makes any modification to any of their roadside safety hardware that has an existing eligibility letter from FHWA, the manufacturer must notify FHWA of such modification with a request for continued eligibility for reimbursement. The notice of all modifications to a device must be accompanied by:

- Significant modifications For these modifications, crash test results must be submitted with accompanying documentation and videos.
- Non-signification modifications For these modifications, a statement from the crash test laboratory on the potential effect of the modification on the ability of the device to meet the relevant crash test criteria.

FHWA's determination of continued eligibility for the modified hardware will be based on whether the modified hardware will continue to meet the relevant crash test criteria.

Any user or agency relying on this eligibility letter is expected to use the same designs, specifications, drawings, installation and maintenance instructions as those submitted for review.

Any user or agency relying on this eligibility letter, is expected to ensure that the hardware used has the same chemistry, mechanical properties, and geometry as that submitted for review, and that it will meet the test and evaluation criteria of the AASHTO MASH.

Issuance of this letter does not convey property rights of any sort or any exclusive privilege. This letter is based on the premise that information and reports submitted by you are accurate and correct. We reserve the right to modify or revoke this letter if: (1) there are any inaccuracies in the information submitted in support of your request for this letter, (2) the qualification testing was flawed, (3) in-service performance or other information reveals safety problems, (4) the system is significantly different from the version that was crash tested, or (5) any other information indicates that the letter was issued in error or otherwise does not reflect full and complete information about the crashworthiness of the system.

Standard Provisions

- To prevent misunderstanding by others, this letter of eligibility designated as FHWA control number B-274 shall not be reproduced except in full. This letter and the test documentation upon which it is based are public information. All such letters and documentation may be reviewed upon request.
- This letter shall not be construed as authorization or consent by the FHWA to use, manufacture, or sell any patented system for which the applicant is not the patent holder.
- If the subject device is a patented product it may be considered to be proprietary. If proprietary systems are specified by a highway agency for use on Federal-aid projects: (a) they must be supplied through competitive bidding with equally suitable unpatented items; (b) the highway agency must certify that they are essential for synchronization with the existing highway facilities or that no equally suitable alternative exists; or (c) they must be used for research or for a distinctive type of construction on relatively short sections of road for experimental purposes. Our regulations concerning proprietary products are contained in Title 23, Code of Federal Regulations, Section 635.411.

Sincerely,

Scoto T. Musa

Scott T. Johnson Director, Office of Safety Technologies Office of Safety

Enclosures

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1-1-1

Request for Federal Aid Reimbursement Eligibility of Highway Safety Hardware

	Date of Request:	11-4-2016	• New	⊂ Resubmission		
	Name:	Michael Zdenek, P.E.	chael Zdenek, P.E.			
ter	Company:	HNTB Corporation				
Submitter	Address:	Empire State Building, 56th Floor, New York, NY, 10118				
Sut	Country:	United States of America				
	To:	Michael S. Griffith, Director FHWA, Office of Safety Technologies				

I request the following devices be considered eligible for reimbursement under the Federal-aid highway program.

Device & Testing	Criterion - Enter from right to left starting with Test Level

System Type	Submission Type	Device Name / Variant	Testing Criterion	Test Level
'B': Rigid/Semi-Rigid Barriers (Roadside, Median, Bridge Railings)	 Physical Crash Testing Engineering Analysis 	TBTA Bridge Rail	AASHTO MASH	TL5

By submitting this request for review and evaluation by the Federal Highway Administration, I certify that the product(s) was (were) tested in conformity with the AASHTO Manual for Assessing Safety Hardware and that the evaluation results meet the appropriate evaluation criteria in the MASH.

Individual or Organization responsible for the product:

Contact Name:	Aris Stathopoulos, P.E.	Same as Submitter 🗌	
Company Name:	MTA Bridges and Tunnels	Same as Submitter 🗌	
Address:	2 Broadway, 22nd Floor, New York, NY, 10004	Same as Submitter 🗌	
Country:	United States of America	Same as Submitter 🗌	
Enter below all disclosures of financial interests as required by the FHWA `Federal-Aid Reimbursement Eligibility Process for Safety Hardware Devices' document.			

HNTB: HNTB Corporation is a paid consultant for MTA-TBTA for this project and eligibility request. HNTB has no further financial interest in the use of this barrier system.

TTI: Texas A&M Transportation Institute was contracted by HNTB to perform analysis and full-scale crash testing of the TBTA VN Bridge Rail design.

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PRODUCT DESCRIPTION

6	New Hardware or	M	odification to	
(•	Significant Modification	Ex	isting Hardwar	e

The test installation was a 132 ft long (post to post) steel bridge rail with four rail tubes mounted on 17 posts. The bridge rail was comprised of four HSS steel tubes. The rail measured 3 ft-6 inches in height above the bridge deck, and the posts were equally spaced at 8 ft-3 inches along the length of the installation. The centerlines of the rails were located 40½ inches, 30 inches, 18 inches, and 7½ inches above the paved surface of the bridge deck. Seventeen fabricated steel posts, each 3 ft-7¾ inches in overall height , supported the four rails at equal post spacing of 8 ft-3 inches along the length of the rail. Each railing post was a built up welded structure that was comprised of a W8×28 beam, 3 ft 6 inches tall, that was beveled at the top 1¾-inch downward to the field side.

The first two posts (right to left) were attached to a concrete foundation. Posts 3-9 were attached to a 49 ft-6 inch long surrogate composite bridge span. Posts 10-17 were also mounted on a concrete foundation. Since the surrogate deck adds significant cost to testing, the bridge deck was limited to a length that would take most of the loading from the vehicle impact. The remaining length of the rail, which sustains much less load, was attached to a concrete foundation to reduce installation cost.

CRASH TESTING

By signature below, the Engineer affiliated with the testing laboratory, agrees in support of this submission that all of the critical and relevant crash tests for this device listed above were conducted to meet the MASH test criteria. The Engineer has determined that no other crash tests are necessary to determine the device meets the MASH criteria.

Engineer Name:	Nauman Sheikh, P.E.		
Engineer Signature:	Nauman Sheikh	Digitally signed by Naum DN: cn=Nauman Sheikh, email=n-sheikh@tti.tamu Date: 2016.11.07 16:44:19	p=Texas A&M Transportation Institute, ou, .edu, c=US
Address:	TTI, TAMUS MS 3135, College Station, T	X 77843-3135	Same as Submitter 🗌
Country:	USA		Same as Submitter 🗌

A brief description of each crash test and its result:

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Required Test Number	Narrative Description	Evaluation Results
5-10 (1100C)	Test 603911-1, June 7, 2016, Report 603911-1-3 A 2010 Kia Rio test vehicle, traveling at an impact speed of 62.5 mi/h, contacted the the TBTA Bridge Rail 3.1 ft upstream of the splice between posts 4 and 5 at an impact angle of 24.7 degrees. The TBTA Bridge Rail contained and redirected the 1100C vehicle. The vehicle did not penetrate, underride, or override the installation. Maximum dynamic deflection during the test was 1.5 inches. No detached elements, fragments, or other debris were present to penetrate or show potential for penetrating the occupant compartment, or to present hazard to others in the area. Maximum occupant compartment deformation was 2.25 inches in the right front floor pan area and 2.0 inches in the right front firewall area. The 1100C vehicle remained upright during and after the collision event. Maximum roll and pitch angles were 9 degrees and 8 degrees, respectively. Occupant risk factors were within the limits specified in MASH. The TBTA steel bridge rail performed acceptably for MASH Test 5-10.	PASS
5-11 (2270P)	Test 603911-2, June 9, 2016, Report 603911-1-3 A 2010 Dodge RAM 1500 pickup truck, traveling at an impact speed of 64.3 mi/h, contacted the TBTA steel bridge rail 4.0 ft upstream of the splice between posts 4 and 5 at an impact angle of 24.8 degrees. The TBTA Bridge Rail contained and redirected the 2270P vehicle. The vehicle did not penetrate, underride, or override the installation. Maximum dynamic deflection during the test was 2.0 inches. No detached elements, fragments, or other debris were present to penetrate or show potential for penetrating the occupant compartment, or to present hazard to others in the area. Maximum occupant compartment deformation was 5.0 inches in the right front firewall area. The 2270P vehicle remained upright during and after the collision event. Maximum roll and pitch angles were 10 degrees and 4 degrees, respectively. Occupant risk factors were within the limits specified in MASH. The TBTA steel bridge rail performed acceptably for MASH Test 5-11.	PASS

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		Fage 4 OF 5
Required Test Number	Narrative Description	Evaluation Results
5-12 (36000V)	Test 603911-3, June 17, 2016, Report 603911-1-3 A 2006 International 8600 tractor with 1997 Stoughton AVW 5357-S-C-AR van trailer, traveling at an impact speed of 49.9 mi/h, contacted the bridge rail 6.0 inches downstream of the splice between posts 4 and 5 at an impact angle of 15.1 degrees. The TBTA Bridge Rail contained and redirected the 36000V vehicle. The vehicle did not penetrate, underride, or override the installation. Maximum dynamic deflection during the test was 2.0 inches. No detached elements, fragments, or other debris were present to penetrate or show potential for penetrating the occupant compartment, or to present hazard to others in the area. No occupant compartment deformation or intrusion was noted. The 36000V vehicle remained upright during and after the collision event. Maximum roll and pitch angles were 11 degrees and 14 degrees, respectively. The TBTA steel bridge rail performed acceptably for MASH Test 5-12.	PASS
5-20 (1100C)	Device is not a transition	Non-Relevant Test, not conducted
5-21 (2270P)	Device is not a transition	Non-Relevant Test, not conducted
5-22 (36000V)	Device is not a transition	Non-Relevant Test, not conducted

Full Scale Crash Testing was done in compliance with MASH by the following accredited crash test laboratory (cite the laboratory's accreditation status as noted in the crash test reports.):

Laboratory Name:	Texas AM Transportation Institute		
Laboratory Signature:	Darrell R. Kuhn DN: Gro	Digitally signed by Darrell L. Kuhn DN: cn=Darrell L. Kuhn, o=Texas A&M Transportation Insitute, ou=Prc Ground, email=d-kuhnetitamu.edu, c=US Date: 2016.11.08 10:15:53 -06:00'	
Address:	TTI, TAMUS MS 3135, College Station, TX 77843-3135		Same as Submitter 🗌
Country:	USA		Same as Submitter 🗌
Accreditation Certificate Number and Dates of current Accreditation period :	ent Certificate Number: 2821.01 Valid To: April 30, 2017		

Submitter Signature*: Michael Zdenek Digitally signed by Michael Zdenek Date: 2016.11.09 09:34:39 -05'00'

Submit Form

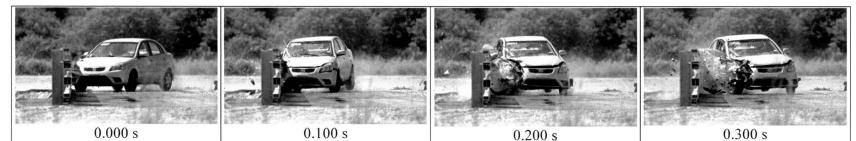
ATTACHMENTS

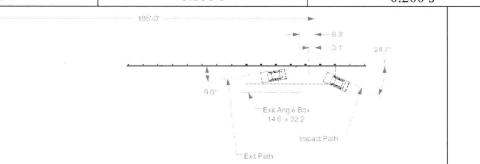
Attach to this form:

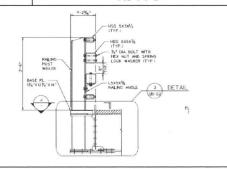
- 1) Additional disclosures of related financial interest as indicated above.
- 2) A copy of the full test report, video, and a Test Data Summary Sheet for each test conducted in support of this request.
- 3) A drawing or drawings of the device(s) that conform to the Task Force-13 Drawing Specifications [Hardware Guide Drawing Standards]. For proprietary products, a single isometric line drawing is usually acceptable to illustrate the product, with detailed specifications, intended use, and contact information provided on the reverse. Additional drawings (not in TF-13 format) showing details that are relevant to understanding the dimensions and performance of the device should also be submitted to facilitate our review.

FHWA Official Business Only:

Eligibility Letter			
Number	Date	Key Words	







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General Information Impact Conditions Post-Impact Trajectory Stopping Distance..... 185 ft dwnstrm Test Agency..... Texas A&M Transportation Institute (TTI) Angle 24.7 degrees Test Standard Test No..... MASH Test 5-10 Vehicle Stability TTI Test No. 603911-1 Location/Orientation 3.1 ft upstrm of Test Date 2016-06-07 splice btw 4 & 5 Test Article Impact Severity..... 55 kip-ft Type Bridge Rail **Exit Conditions** Name..... TBTA Bridge Rail Installation Length..... 132 ft long (post to post) Speed 48.3 mi/h Material or Key Elements ... Quadruple rail steel bridge rail 3 ft-6 Angle 9.6 degrees inches in height mounted on 17 posts **Occupant Risk Values Test Article Deflections** attached either to a 49 ft-6 inch bridge Longitudinal OIV 22.0 ft/s span (posts 3-9), or to a concrete Lateral OIV...... 34.8 ft/s Longitudinal Ridedown 4.1 g foundation up to the bridge span and beyond the bridge span Lateral Ridedown 10.9 g Soil Type and Condition Concrete Bridge Deck THIV 44.8 km/h Vehicle Damage **Test Vehicle** PHD..... 10.9 g Type/Designation..... 1100C Make and Model 2010 Kia Rio Max. 0.050-s Average Curb..... 2478 lb Longitudinal -13.1 g Test Inertial 2425 lb Lateral.....-21.2 g Dummy 165 lb Vertical.....-3.2 g Gross Static 2590 lb

6.5 ft twd traffic

Maximum Yaw Angle 74 degree	S
Maximum Pitch Angle 8 degrees	
Maximum Roll Angle 9 degrees	
Vehicle Snagging No	
Vehicle Pocketing No	

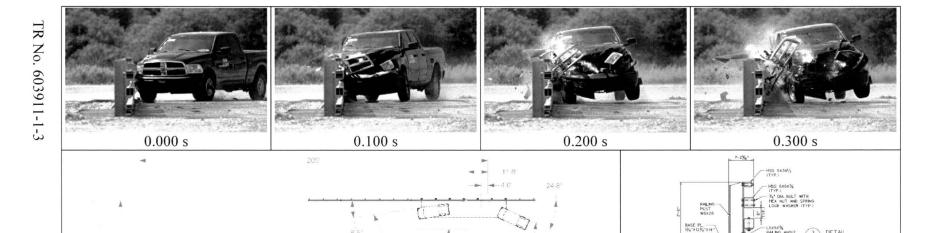
Dynamic	1.5 inches
Permanent	0.5 inch
Working Width	15.5 inches

VDS	01RFQ4
CDC	01FREW3
Max. Exterior Deformation	10.5 inches
OCDI	RF0013000
Max. Occupant Compartment	
Deformation	2.25 inches

Figure 5.7. Summary of Results for MASH Test 5-10 on TBTA Bridge Rail.

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TR No. 603911-1-3



L5X5X% RALING ANGLE

- Hard Labor

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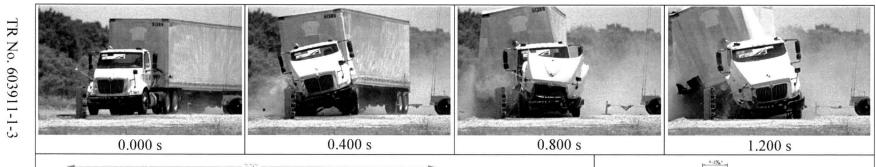
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80	General Information		Impact Conditions	Post-Impact Trajectory
	Test Agency	Texas A&M Transportation Institute (TTI)	Speed 64.3 mi/h	Stopping Distance 205 ft dwnstrm
	Test Standard Test No		Angle 24.8 degrees	50 ft twd traffic
	TTI Test No.	603911-2	Location/Orientation 4.0 ft upstream	of Vehicle Stability
	Test Date	2016-06-09	splice btw 4 and	1 5 Maximum Yaw Angle 43 degrees
	Test Article		Impact Severity 123 kip-ft	Maximum Pitch Angle 4 degrees
	Туре	Bridge Rail		Maximum Roll Angle 10 degrees
	Name		Exit Conditions	Vehicle Snagging No
	Installation Length		Speed 51.9 mi/h	Vehicle Pocketing No
		Quadruple rail steel bridge rail 3 ft-6	Angle 8.5 degrees	· ·
	(1995) Characterization (American Control of Control	inches in height mounted on 17 posts	Occupant Risk Values	Test Article Deflections
		attached either to a 49 ft-6 inch bridge	Longitudinal OIV 17.4 ft/s	Dynamic 2.0 inches
		span (posts 3-9), or to a concrete	Lateral OIV 28.5 ft/s	Permanent 0.75 inch
		foundation up to the bridge span and	Longitudinal Ridedown 6.0 g	Working Width 15.8 inches
		beyond the bridge span	Lateral Ridedown 10.7 g	
	Soil Type and Condition	Concrete Bridge Deck	THIV	Vehicle Damage
	Test Vehicle		PHD 10.8 g	VDS 01RFQ4
	Type/Designation	2270P	ASI 1.92	CDC 01FREW4
)		2010 Dodge RAM 1500 Pickup Truck	Max. 0.050-s Average	Max. Exterior Deformation 16.0 inches
>	Curb		Longitudinal8.5 g	OCDI RF0030000
_	Test Inertial	5052 lb	Lateral15.2 g	Max. Occupant Compartment
>	Dummy	165 lb	Vertical2.8 g	Deformation 5.0 inches
c 00 7100	Gross Static		V	
2		Figure 6.7 Summary of R	esults for MASH Test 5-11 on TB	TA Bridge Rail

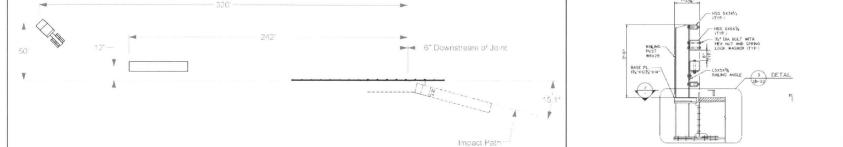
Exit Path

Fxit Angle Box 16.6' x 32.2

Figure 6.7. Summary of Results for *MASH* Test 5-11 on TBTA Bridge Rail.

2016-09-30





General Information		Impact Conditions		Post-Impact Trajecto
Test Agency	Texas A&M Transportation Institute (TTI)	Speed	. 49.9 mi/h	Stopping Distance.
Test Standard Test No	MASH Test 5-12	Angle	15.1 degrees	
TTI Test No.	603911-3	Location/Orientation		trm Vehicle Stability
Test Date	2016-06-17		of splice btw 4 &	5 Maximum Yaw Ang
Test Article		Impact Severity	450 kip-ft	Maximum Pitch Ang
Туре	Bridge Rail		a contra confirma	Maximum Roll Ang
Name		Exit Conditions		Vehicle Snagging
Installation Length	132 ft long (post to post)	Speed	. 43.1 mi/h	Vehicle Pocketing
	Quadruple rail steel bridge rail 3 ft-6	Angle		3
	inches in height mounted on 17 posts	Occupant Risk Values	0	Test Article Deflection
	attached either to a 49 ft-6 inch bridge	Longitudinal OIV	. 12.1 ft/s	Dynamic
	span (posts 3-9), or to a concrete	Lateral OIV		Permanent
	foundation up to the bridge span and	Longitudinal Ridedown	8.7 g	Working Width
	beyond the bridge span	Lateral Ridedown	. 10.4 g	
Soil Type and Condition	Concrete Bridge Deck	THIV		Vehicle Damage
Test Vehicle		PHD	. 11.0 g	CDC
Type/Designation	36000∨	ASI		Max. Exterior Defor
Make and Model	2006 International 8600 tractor	Max. 0.050-s Average		Max. Occupant Cor
	with 1997 Stoughton AVW 5357-S-C-AR	Longitudinal	-6.6 g	Deformation
	van trailer	Lateral		
Curb	29,870 lb	Vertical	9.1 g	
Gross Static & Inertial	79,620 lb			
	Elauna 7.0 Summary of D	and the fam MACILT.	5 12 TD	TA Dullas Dall

ctory

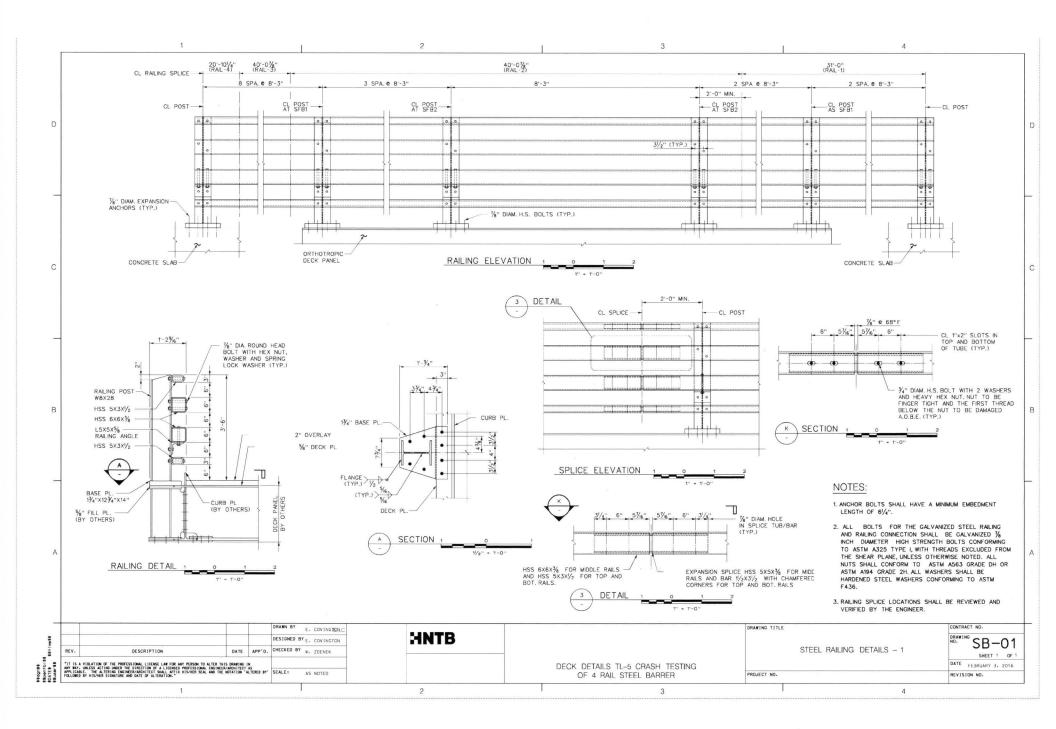
Stopping Distance	320 downstream 50 ft twd traffic
ehicle Stability	
Maximum Yaw Angle	40 degrees
Maximum Pitch Angle	14 degrees
Maximum Roll Angle	
Vehicle Snagging	No
Vehicle Pocketing	

ctions

Dynamic	2.0 inches
Permanent	0.6 inches
Working Width	62.0 inches

CDC	
Max. Exterior Deformation	20.0 inches
Max. Occupant Compartment	
Deformation	None

Figure 7.9. Summary of Results for MASH Test 5-12 on TBTA Bridge Rail.





Texas A&M Transportation Institute 3135 TAMU College Station, TX 77843-3135

979-845-6375 Fax: 979-845-6107 http://tti.tamu.edu/crashtesting

Test Report No. 603911-1-3 Test Report Date: September 2016

Crash Test No. 603911-3 – *MASH* Test 2-12 RE: Fuel Tank Damage

Excerpt from report:

7.6 VEHICLE DAMAGE

Figures 7.6 through 7.8 shows the damage sustained by the vehicle. The front bumper, hood, front axle, right front springs and U-bolts, right front tire and rim, right fuel tank, and right steps of the tractor were damaged. Maximum exterior crush to the tractor was 20.0 inches in the side plane at the right front corner at bumper height. No occupant compartment deformation or intrusion was noted. Figure 7.8 shows the interior of the vehicle. The trailer broke apart near the fifth wheel and all of the tires and rims on the right side were damaged.

TTI Proving Ground Response 2017-01-17:

The fuel tank was only deformed/dented. No punctures or seam ruptures were noted.

If additional information is needed, please contact

Wander L. Menges

Wanda L. Menges Research Specialist TTI Proving Ground

TTI Proving Ground 3100 SH 47, Bldg. 7091 Bryan, TX 77807