



US Department  
of Transportation  
**Federal Highway  
Administration**

400 Seventh St., S.W.  
Washington, D.C. 20590

August 27, 1999

Refer to: HMHS-CC61

Mr. Kaddo Kothmann  
President  
ROAD SYSTEMS, INC.  
P.O. Box 2163  
Big Spring, Texas 79721

Dear Mr. Kothmann:

In your July 30 letter, you requested the Federal Highway Administration's (FHWA) acceptance of a steel breakaway post as an alternative to the weakened timber posts that are currently used in your SKT-350 and FLEAT-350 w-beam guardrail terminals. These breakaway posts are comprised of a lower stub post connected to an upper post by splice plates welded to the flanges of the stub post along the bottom and sides of the plates and connected to the upper post with two 31-mm diameter plug welds. This design causes the plug welds to yield at relatively low loads when the posts are struck head on and the welds are loaded in torsion, but the connection can sustain loads as high as 89 KN when loaded laterally in shear. Enclosure 1 shows the breakaway end posts, the breakaway line posts, and the splice weld details. All other features of the SKT-350 and the FLEAT-350 remain unchanged from the original designs.

To show that the steel breakaway posts functioned as desired, you ran three tests on the alternative design, and provided me with copies of the test reports for staff review. Summaries of each of the tests are shown in Enclosure 2.

We believe that the tests you ran satisfactorily demonstrate that the steel breakaway posts are an acceptable alternative to the original wood post designs for the SKT-350 and the FLEAT-350 and may be used as such on the National Highway System when requested by a transportation agency.

Sincerely yours,

Dwight A. Horne  
Director, Office of Highway Safety Infrastructure

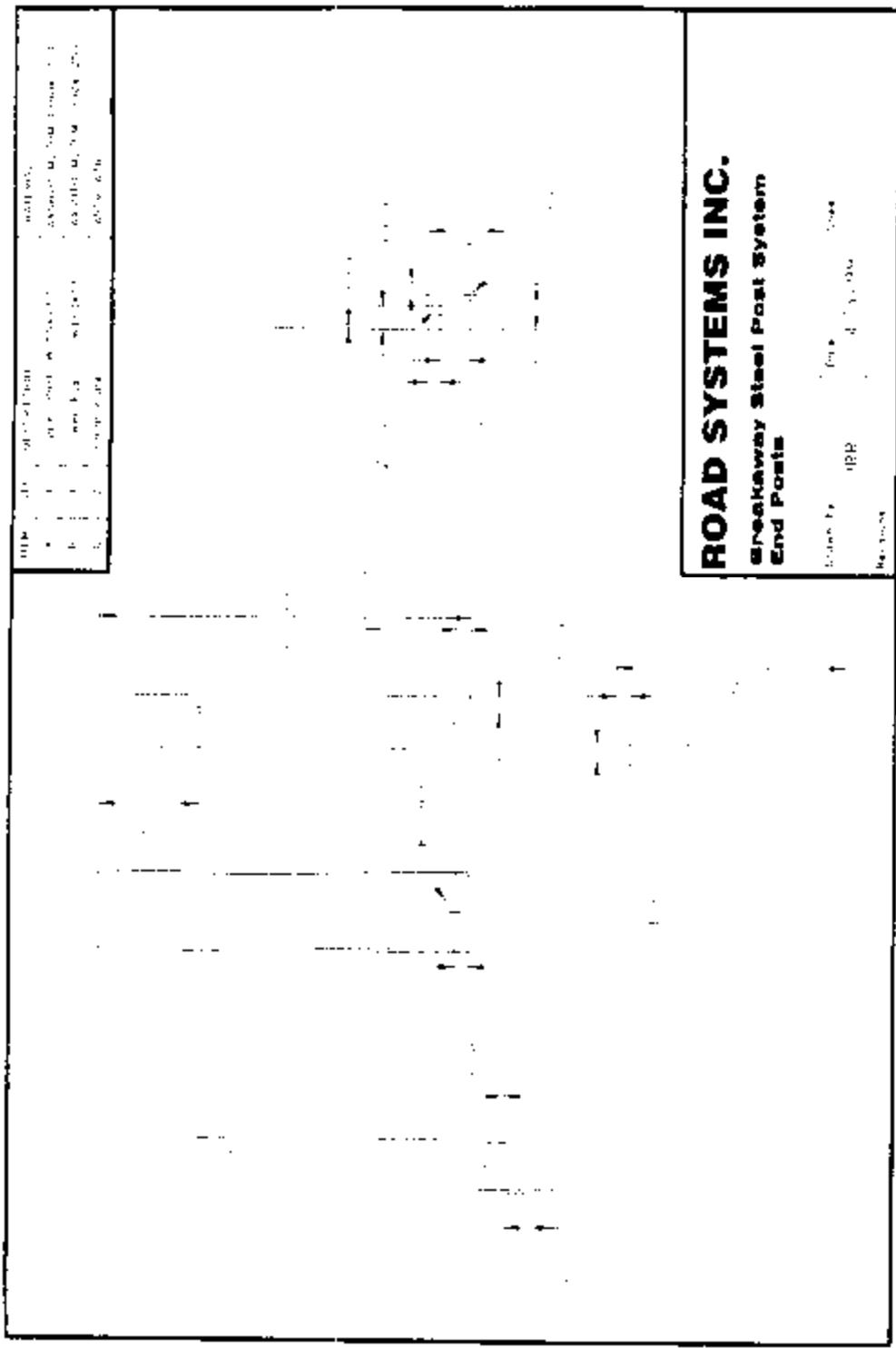


Figure 9. Steel Breakaway End Post.

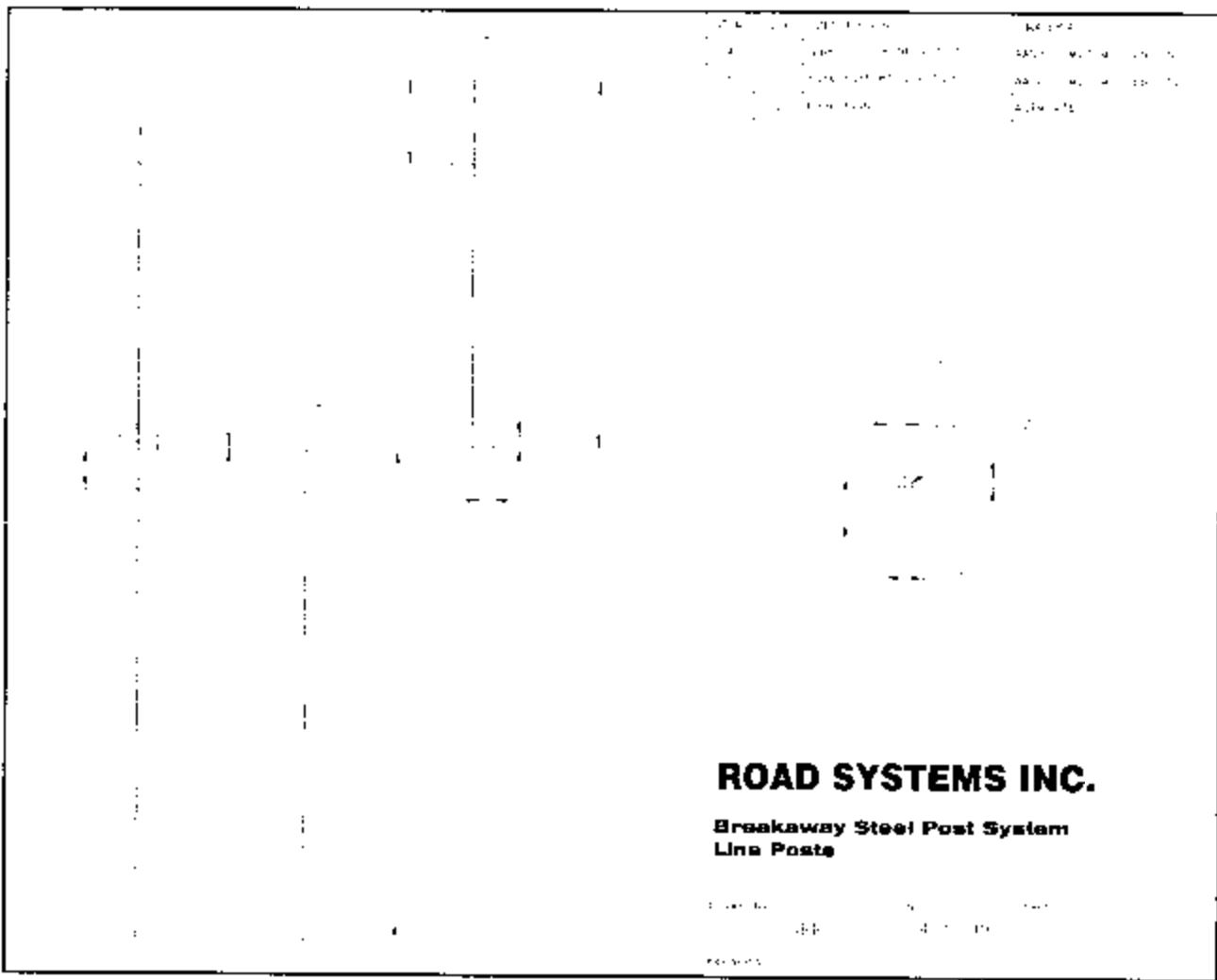
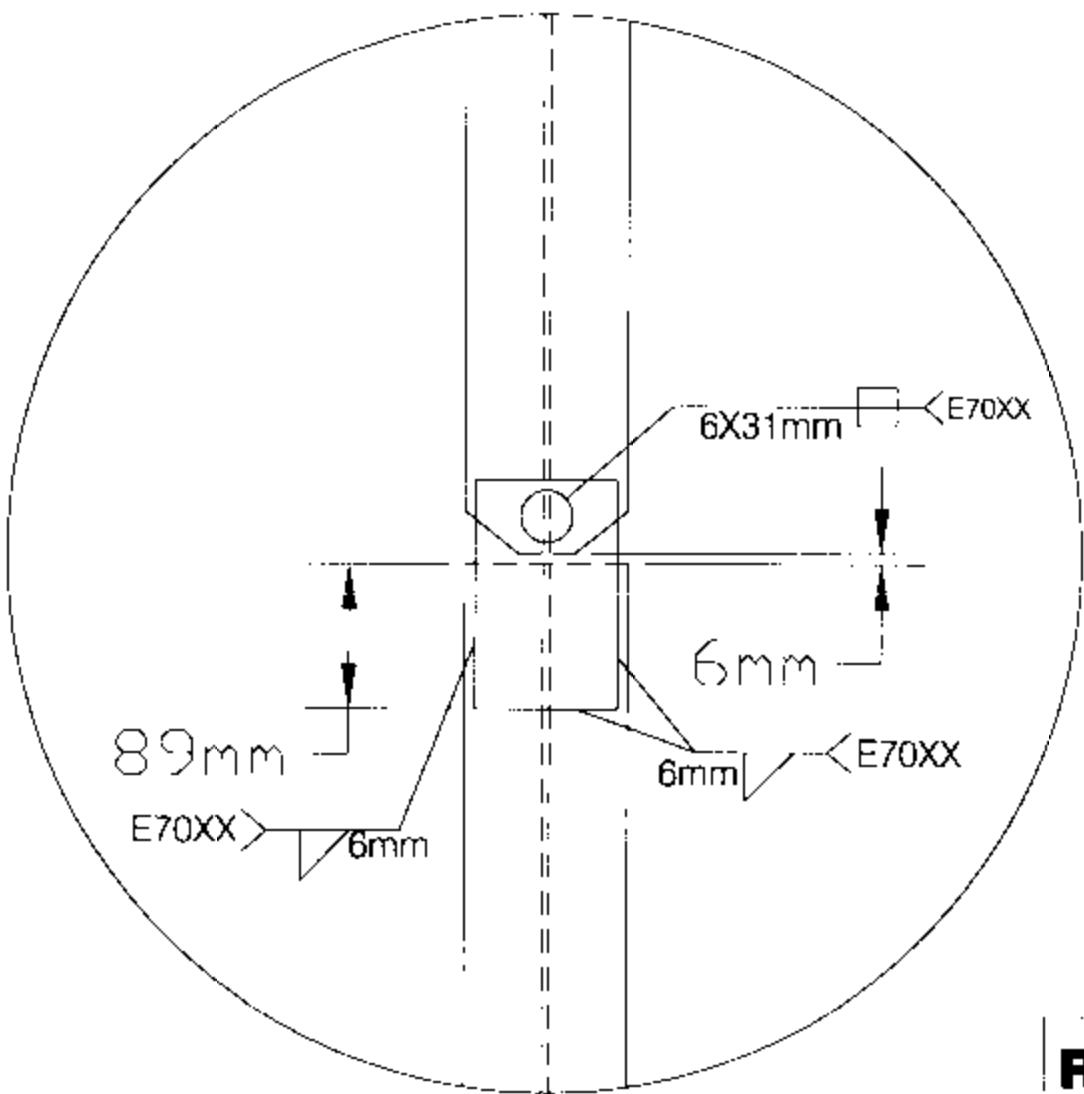


Figure 11. Steel Breakaway Line Post.

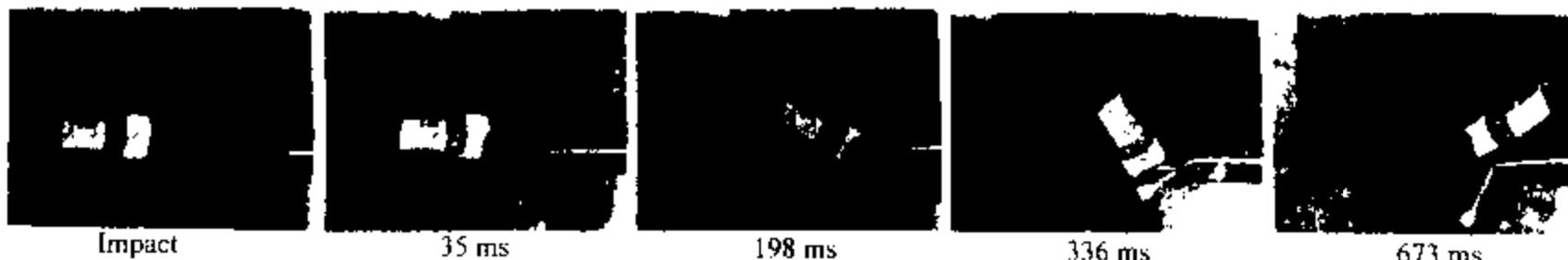


Note:  
Weld typical on both flanges  
of post.

## ROAD SYSTEMS INC.

### Steel Line Posts Weld Details

Drawn by	JAA	Date	Sheet
Revisions		6/29/99	



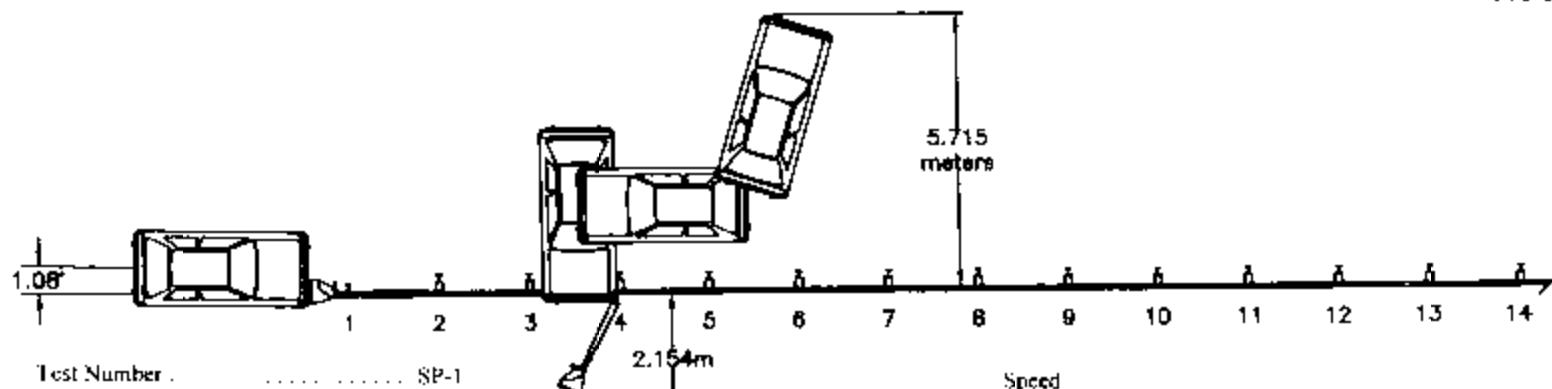
Impact

35 ms

198 ms

336 ms

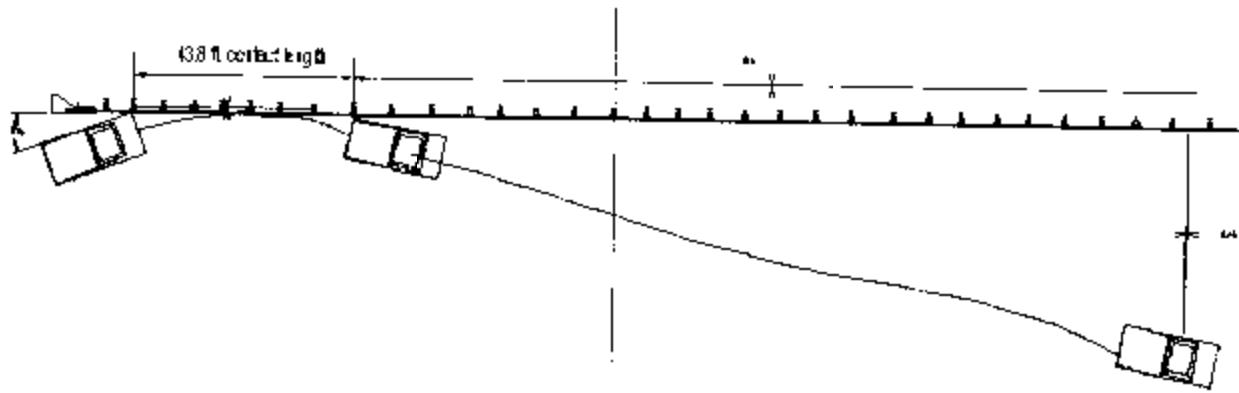
673 ms



Test Number .....	SP-1	Speed	Impact .....	98.64 km/h
NCHRP 350 Test Designation .....	3-31	Exit .....	N.A.	
Date .....	2/2/98	Angle	Impact .....	1.08 deg (on head)
Installation .....	Sequential Kinking Terminal	Exit .....	N.A.	
System length .....	41.91 m	Occupant Impact Velocity	Longitudinal .....	8.40 m/s
Head Dimensions (LxWxH) .....	2104 mm x 508 mm x 508 mm	Lateral .....	3.51 m/s	
Face Angle .....	0.0 degrees	Occupant Ridedown Deceleration	Longitudinal .....	12.12 g's
Guardrail .....	12-gauge W-beam	Lateral .....	10.81 g's	
End Terminal Posts		Vehicle Damage	TAD .....	12-FR-3
Number 1 .....	W150x13.5 BCT steel post, 1080 mm long in SKT foundation tube w/ modified groundline strut to post 2	VDI .....	12FREN2	
Number 2 .....	W150x13.5 BCT steel post, 1830 mm long in SKT foundation tube w/ modified groundline strut and 200x150x360 mm routered timber block	Vehicle Rebound Distance .....	5.7 meters (approx.)	
Numbers 3-8 .....	W150x13.5 CRT steel posts, 1830 mm long w/ 200x150x360 mm routered timber block			
Vehicle Model .....	1993 Ford Festiva Compact Car			
Vehicle Weight				
Curb .....	795 kg			
Test Inertia .....	819 kg			
Gross Static .....	894 kg			

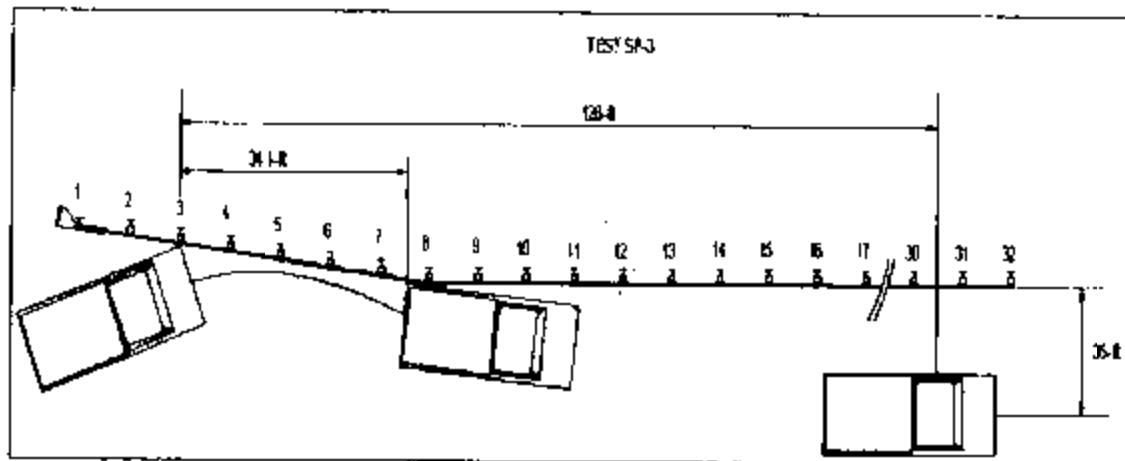
Conversion Factors: 1 in. = 2.54 cm, 1 lb = 0.454 kg

Figure 17. Summary of Test SP-1



General Information		Test Vehicle (continued)		Ridedown Accelerations (g's)	
Test Agency	Southwest Research Institute	Mass (kg) Dummy(s)	75.0	Y-direction	12.6
Test Number	SP-2	Mass (kg) Gross Statis.	2030.0	Test Article Deflection (mm)	
Test Date	Feb 18, 1999	Impact Conditions		Dynamic	1169
Test Article	Guardrail End Terminal	Speed (km/h)	100.1	Permanent	935
Type	Sequential Kinking Terminal	Angle (deg)	20.0	Vehicle Damage	
Installation Length (m)	61	Exit Conditions		Exterior	
Barrier	W-beam	Speed (km/h)	64.9	VDS	11FO
Soil Type and Condition	S1 Dry	Angle (deg)	15.8	DCDC	11FNEN
Test Vehicle		Occupant Risk Values		Interior	
Type	Standard Pickup	Impact Velocity (m/s)		OCDI	LF000000
Designation	2000P	X-direction	3.8	Post-Impact Vehicular Behavior	
Model	1993 Chevrolet C-20	Y direction	3.5	Maximum Roll Angle (deg)	7.3
Mass (kg) Curb	1925	Ridedown Accelerations (g's)		Maximum Pitch Angle (deg)	4.5
Mass (kg) Test Inertial	1925	X-direction	10.5	Maximum Yaw Angle (deg)	Not Available

Figure 13. Impact Description and Summary of Results, Test SP-2



General Information		Test Vehicle (continued)		Rollover Accelerations (g's)	
Test Agency	Southwest Research Institute	Mass (kg) Dummy(s)	75.0	Y-direction	7.8
Test Number	SP-3	Mass (kg) Gross Static	2020.0	Test Article Deflection (mm)	
Test Date	13-Apr-99	Impact Conditions		Dynamic	1217
Test Article	4-ft Offset Guardrail End Terminal	Speed (km/h)	104.5	Permanent	1080
Type	Sequential Kinking Terminal	Angle (deg)	20.4	Vehicle Damage	
Installation Length (m)	61	Exit Conditions		Exterior	
Barrier	W-beam	Speed (km/h)	55.1	VDS	11FQ-2
Soil Type and Condition	S1-Dry	Angle (deg)	16.0	DCDC	11FNEN
Test Vehicle		Occupant Risk Values		Interior	
Type	Standard Pickup	Impact Velocity (m/s)		OCDI	LFD000000
Designation	2000P	X-direction	5.5	Post-Impact Vehicular Behavior	
Model	1993 Chevrolet C-20	Y-direction	3.6	Maximum Roll Angle (deg)	5
Mass (kg) Curb	1945	Rollover Accelerations (g's)		Maximum Pitch Angle (deg)	9.4
Mass (kg) Test Inertial	1945	X-direction	7.5	Maximum Yaw Angle (deg)	Not Available

Figure 14. Impact Description and Summary of Results, Test SP-3